

APPENDIX B
STRUCTURAL ENGINEERING DESIGN AND CRITERIA

1.0 INTRODUCTION

This appendix summarizes the codes, standards, criteria, and practices that will be generally used in the design and construction of structural engineering systems for the Pio Pico Energy Center (PPEC). More specific project information will be developed during execution of the project to support detailed design, engineering, material procurement specification, and construction specifications.

2.0 CODES AND STANDARDS

The design of structural engineering systems for PPEC will be in accordance with the applicable laws, ordinances, and regulations of the federal government, State of California, and local agencies, as well as industry standards. The current issue or edition of the documents at the time of filing this Application for Certification will apply, unless otherwise noted. In cases where conflicts exist between the cited documents, requirements of the more conservative document will be used.

The following codes and standards have been identified as applicable, in whole or in part, to structural engineering design and construction of power plants:

- American Institute of Steel Construction (AISC)
 - Manual of Steel Construction - 13th Edition
 - Specification for Structural Steel Buildings
 - Specification for Structural Joints Using American Society for Testing Materials (ASTM) A325 or A490 Bolts
 - Code of Standard Practice for Steel Buildings and Bridges
- American Concrete Institute (ACI)
 - ACI 318, Building Code Requirements for Structural Concrete
 - ACI 301, Specifications for Structural Concrete for Buildings
 - ACI 350R, Environmental Engineering Concrete Structures
 - ACI 543R, Design, Manufacture, and Installation of Concrete Piles
- American Society of Civil Engineers (ASCE)
 - ASCE 7 - Minimum Design Loads for Buildings and Other Structures
- American Welding Society (AWS)
 - D1.1 - Structural Welding Code - Steel
 - D1.3 - Structural Welding Code - Sheet Steel

- California Building Code (CBC)
- Code of Federal Regulations, Title 29 - Labor, Chapter XVII, Occupational Safety and Health Administration (OSHA)
 - Part 1910 - Occupational Safety and Health Standards
 - Part 1926 - Construction Safety and Health Regulations
- Design of Large Steam Turbine-Generator Foundations, ASCE 1987
- Hoist Manufacturers Institute (HMI), Standard Specifications for Electric Wire Rope Hoists (HMI 100)
- National Association of Architectural Metal Manufacturers (NAAMM) - Metal Bar Grating Manual
- National Electric Safety Code (NESC)
- National Fire Protection Association (NFPA Standards)
 - NFPA 850 Fire Protection for Electric Generating Plants
- Steel Deck Institute (SDI) - Design Manual for Floor Decks and Roof Decks

3.0 CEC SPECIAL REQUIREMENTS

Prior to the start of any increment of construction, the proposed lateral-force procedures for PPEC structures and the applicable designs, plans, and drawings for structures will be submitted for approval.

Proposed lateral-force procedures, designs, plans, and drawings shall be those for:

- Major PPEC structures
- Major foundations, equipment supports, and anchorage
- Large, field-fabricated tanks
- Turbine-generator pedestal
- Switchyard structures

4.0 STRUCTURAL DESIGN CRITERIA

4.1 TOPOGRAPHIC ELEVATIONS

Project site topographic elevations will be based on an elevation survey conducted using known elevation benchmarks.

4.2 FROST PENETRATION

The project site is in an area free of frost penetration. Bottom elevation of all foundations for structures and equipment, however, will be maintained at a minimum of 12 inches below the finished grade.

4.3 TEMPERATURES

The design basis temperatures for civil and structural engineering systems will be as follows:

- Maximum: 114°F
- Minimum: 17°F

4.4 DESIGN LOADS**4.4.1 General**

Design loads for structures and foundations will comply with all applicable building code requirements.

4.4.2 Dead Loads

Dead loads will consist of the weights of the structure and all equipment of a permanent or semi-permanent nature, including tanks, bins, wall panels, partitions, roofing, drains, piping, cable trays, bus ducts, and the contents of tanks and bins measured at full operating capacity. The contents of the tanks and bins, however, will not be considered effective in resisting structure uplift due to wind forces, but they will be considered effective for seismic forces.

4.4.3 Live Loads

Live loads will consist of uniform floor live loads and equipment live loads. Uniform live loads are assumed equivalent unit loads that are considered sufficient to provide for movable and transitory loads, such as the weights of people, portable equipment and tools, small equipment or parts (which may be moved over or placed on the floors during maintenance operations), and planking. The uniform live loads will not be applied to floor areas that will be permanently occupied by equipment.

Lateral earth pressures, hydrostatic pressures, and wheel loads from trucks will be considered live loads.

Uniform live loads will be in accordance with ASCE 7, but will not be less than the following:

- Roofs: 20 pounds per square foot (psf)
- Floors and Platforms (steel grating and checkered plates): 100 psf

Additionally, a uniform load of 50 psf will be used to account for piping and cable trays; however, actual loads will be used where the piping and cable loads exceed 50 psf.

Furthermore, a concentrated load of 5 kilo pounds (kips) will be applied concurrently to the floors' supporting beams to maximize stresses in the members, but the reactions from the concentrated loads will not be carried to the columns.

- Floors (elevated concrete floors): 100 psf.

In addition, elevated concrete slabs will be designed to support an alternate concentrated load of 2 kips in lieu of the uniform loads, whichever governs. The concentrated load will be treated as uniformly distributed load acting over an area of 2.5 square feet and will be located in a manner to produce the maximum stress conditions in the slabs.

- Control Room Floor: 150 psf
- Stairs, Landings, and Walkways: 100 psf

In addition, a concentrated load of 2 kips will be applied concurrently to the supporting beams for the walkways to maximize the stresses in the members, but the reactions from the concentrated loads will not be carried to the columns.

- Pipe Racks: 100 psf

Where the piping and cable tray loads exceed the design uniform load, the actual loads will be used. In addition, a concentrated load of 15 kips will be applied concurrently to the supporting beams for the walkways to maximize the stresses in the members, but the reactions from the concentrated loads will not be carried to the columns.

- Hand Railings

Hand railings will be designed for a 200-pound concentrated load applied at any point and in any direction.

- Slabs on Grade: 250 psf
- Truck Loading Surcharge Adjacent to Structures: 500 psf during normal operation and 1,000 psf during construction
- Truck Support Structures: American Association of State Highway and Transportation Officials (AASHTO)-HS20-44
- Special Loading Conditions: Actual loadings

Lay down loads from equipment components during maintenance and floor areas where trucks, forklifts, or other transports will have access will be considered in the design live loads.

Live loads may be reduced in accordance with the provisions of CBC Section 1607.

Posting of the floor load capacity signs for all roofs, elevated floors, platforms, and walkways will be in compliance with the Occupational Safety and Health Standard, Walking and Working Surfaces, Subpart D. Floor load capacity for slabs on grade will not be posted.

4.4.4 Earth Pressures

Earth pressures will be in accordance with the recommendations contained in the PPEC geotechnical report.

4.4.5 Groundwater Pressures

Hydrostatic pressures due to groundwater or temporary water loads will be considered.

4.4.6 Wind Loads

The wind forces will be calculated in accordance with CBC, Chapter 16, with a basic wind speed of 80 miles per hour (mph) and a “C” exposure category.

4.4.7 Seismic Loads

Structures will be designed and constructed to resist the effects of earthquake loads as determined in CBC, Chapter 16. The occupancy category of the structure is III (Special Occupancy Structure), and the corresponding importance factor (I) shall be as specified in CBC. Other seismic parameters will be obtained from the PPEC geotechnical report.

4.4.8 Snow Loads

Snow loads will not be considered.

4.4.9 Turbine-Generator Loads

The combustion turbine-generator loads for pedestal and foundation design will be furnished by the equipment manufacturers and will be applied in accordance with the equipment manufacturers’ specifications, criteria, and recommendations.

4.4.10 Special Considerations for Steel Stacks

Steel stacks will be designed to withstand the normal and abnormal operating conditions in combination with wind loads and seismic loads and will include the along-wind and across-wind effects on the stacks. The design will meet the requirements of American Society of Mechanical Engineers (ASME)/American National Standards Institute (ANSI) STS1 -2000, “Steel Stacks,” using allowable stress design methods; although, increased allowable stress for wind loads as permitted by AISC will not be used.

4.4.11 Special Considerations for Structures and Loads during Construction

The allowable stresses may be increased by 33 percent for temporary structures, permanent structures left temporarily incomplete to facilitate equipment installations, or temporary loads imposed on permanent structures during construction.

Structural backfill may be placed against walls, retaining walls, and similar structures when the concrete strength attains 80 percent of the design compressive strength (f'_c), as determined by sample cylinder tests. Restrictions on structural backfill, if any, will be shown on the engineering design drawings.

Design restrictions imposed on construction shoring removal that are different from normal practices recommended by the ACI codes will be shown on engineering design drawings.

Metal decking used as forms for elevated concrete slabs will be evaluated to adequately support the weight of concrete plus a uniform construction load of 50 psf, without increase.

5.0 DESIGN BASES

5.1 GENERAL

Reinforced concrete structures will be designed by the strength design method, in accordance with ACI 318, "Building Code Requirements for Structural Concrete."

Steel structures will be designed by the working stress method, in accordance with AISC Specification for the Design, Fabrication, and Erection of Structural Steel for Buildings.

Allowable soil-bearing pressures for foundation design will be in accordance with the "Final Subsurface Investigation and Foundation Report" for the PPEC.

Reinforced concrete for sumps, cooling tower basins, and other structures designed to contain water will meet the requirements of ACI 350.

5.2 FACTORS OF SAFETY

The safety factor for all structures, tanks, and equipment supports will be as follows:

- Against Overturning: 1.5
- Against Sliding: 1.5 for Wind Loads, 1.1 for Seismic Loads
- Against Uplift Due to Wind: 1.5
- Against Buoyancy: 1.25

5.3 ALLOWABLE STRESSES

Calculated stresses from the governing loading combinations for structures and equipment supports will not exceed the allowable limits permitted by the applicable codes, standards, and specifications.

5.4 LOAD FACTORS AND LOAD COMBINATIONS

For reinforced concrete structures and equipment supports using the strength method, the strength design equations will be determined based on CBC, Chapter 16 and 19 and ACI-318. The allowable stress design load combinations of CBC Chapter 16 will be used to assess soil-bearing pressure and stability of structures per CBC Chapter 18, respectively.

Steel-framed structures will be designed in accordance with CBC, Chapter 22, and the AISC Specification for the Structural Steel Buildings. Connections will conform to Research Council on Structural Connections of the Engineering Foundation Specification for Structural Joints. Connections for moment frame structures will conform to the recommendations of Federal Emergency Management Agency (FEMA) Report 350 for seismic connections.

6.0 CONSTRUCTION MATERIALS**6.1 CONCRETE AND GROUT**

The design compressive strength (f'_c) of concrete and grout, as measured at 28 days, will be as follows:

- Electrical Duct Bank Encasement and Lean Concrete Backfill (Class L-1): 2,000 pounds per square inch (psi)
- Structural Concrete (Class S1): 3,000 psi
- Structural Concrete (Class S2): 4,000 psi
- Grout (Class G-1): 5,000 psi

The classes of concrete and grout to be used will be shown on engineering design drawings or indicated in design specifications.

6.2 REINFORCING STEEL

Reinforcing steel bars for concrete will be deformed bars of billet steel, conforming to ASTM A 615, Grade 60.

Welded wire fabric for concrete will conform to ASTM A185.

6.3 STRUCTURAL AND MISCELLANEOUS STEEL

Structural and miscellaneous steel will generally conform to ASTM A36, ASTM A572, or ASTM A992, except in special situations where higher strength steel is required.

High-strength structural bolts, including nuts and washers, will conform to ASTM A325 or ASTM A490.

Bolts other than high-strength structural bolts will conform to ASTM 307, Grade A.

6.4 CONCRETE MASONRY

Concrete masonry units will be hollow, normal weight, nonload-bearing Type I conforming to ASTM C 129. Mortar will conform to ASTM C 270, Type M. Grout will conform to ASTM C 476.

6.5 OTHER MATERIALS

Other materials for construction, such as anchor bolts, shear connectors, concrete expansion anchors, and embedded metal, will conform to industry standards and will be identified on engineering design drawings or specifications.